

US Army Corps of Engineers
Huntington District

Engineering Geology Challenges During Design and Construction of the Marmet Lock Project

2005 Tri-Service Infrastructure Systems Conference August 2-5, 5005

Ron Adams & Mike Nield



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- 1. PROJECT OVERVIEW
 - a. Site Plan
 - **b.** Site Geology



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- 1. PROJECT OVERVIEW
 - a. Site Plan
 - **b.** Site Geology
- 2. DEEP SEATED SLIDING
 - a. Design Concerns
 - b. Cofferdam Foundation Movement
 - c. New Chamber Lockwall Monoliths



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Main Topics of Discussion

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3. GEOLOGIC ASPECTS OF CONSTRUCTION

- a. Anchor Installation
- **b.** Rock Excavation
- c. Foundation Preparation & Treatment
- d. Drilled Shaft Foundations
- e. Foundation Drilling and Grouting

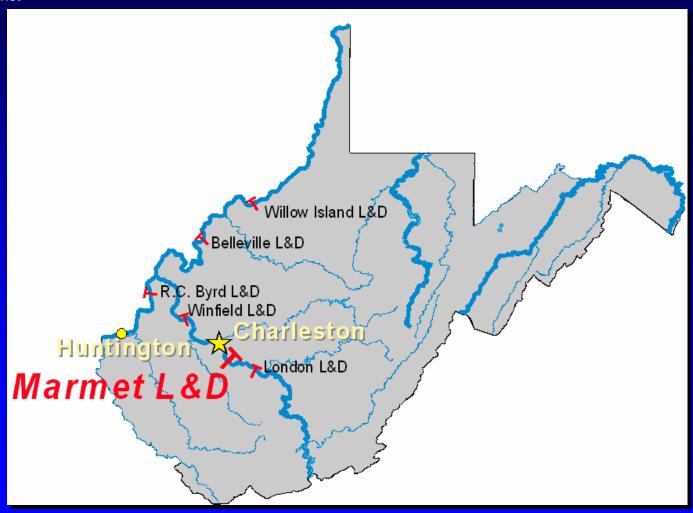


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- 1. PROJECT OVERVIEW
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Site Plan



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Marmet Lock Replacement Project

 Purpose is to alleviate traffic on the Kanawha River



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Huntington District Marmet Lock Replacement Project

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- Existing and smaller 56' X 360' lock chambers inhibit river traffic



Huntington District Marmet Lock Replacement Project

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- Project adds one larger 110' X 800' lock chamber and approach walls, located landward of old locks



Huntington District Marmet Lock Replacement Project

- Purpose is to alleviate traffic on the Kanawha River
- Existing and smaller 56' X 360' lock chambers inhibit river traffic
- Project adds one larger 110' X 800' lock chamber and approach walls, located landward of old locks
- **Construction started in summer 2002** with a current contract cost of \$232 Million



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Existing Lock – Prior to Construction



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New Lock - Conceptual Drawing



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Marmet Lock Replacement - Project Overview



New Lock Chamber Under Construction

Existing Lock Chambers

New Lock Construction - Ariel View - May 2005



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Sheet Pile Cell Cofferdam

Existing - Landwall Portion of Cofferdam



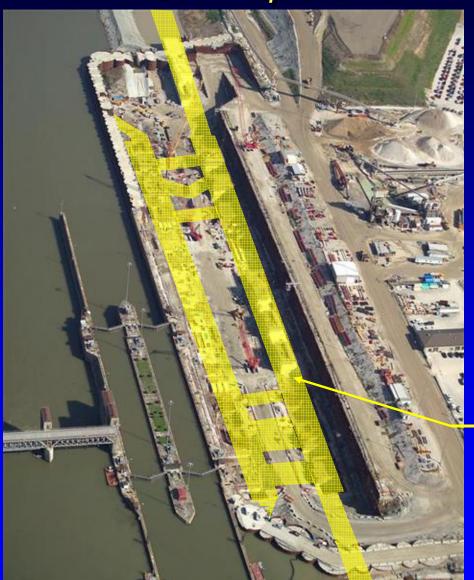
Contractor
Designed
Anchored
Retaining Wall

New Lock Construction - Ariel View - May 2005



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Marmet Lock Replacement – Project Overview



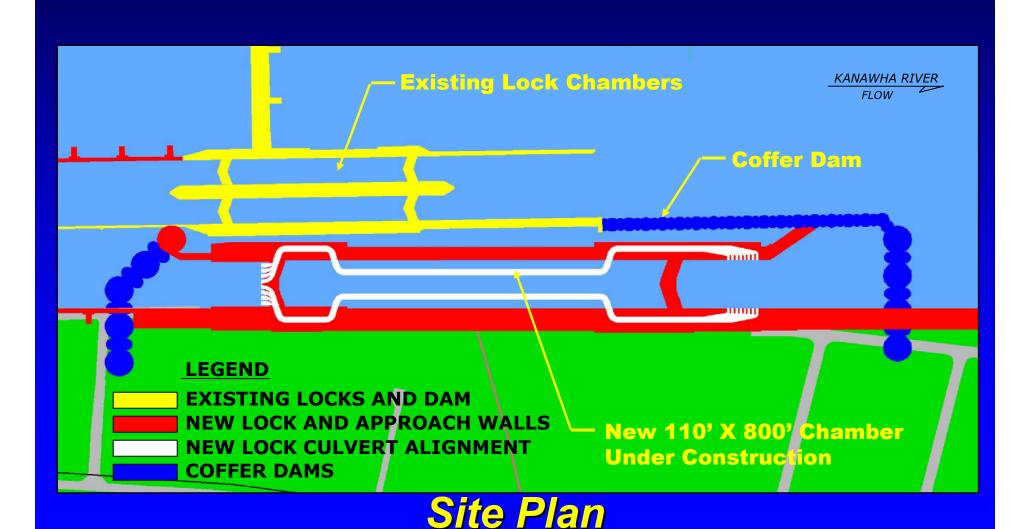
New Lock Chamber and Approach Walls

New Lock Construction - Ariel View - May 2005



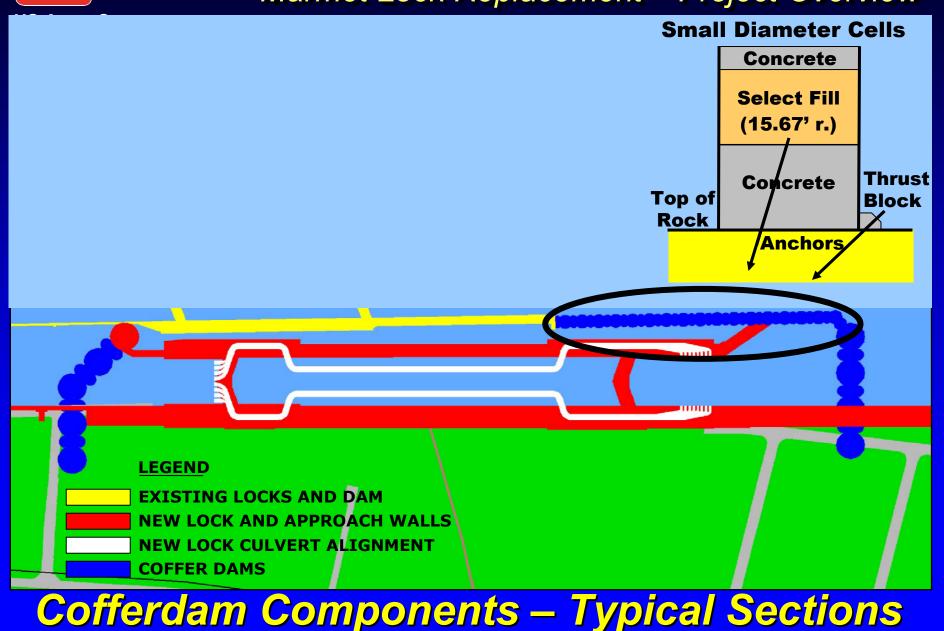
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Marmet Lock Replacement - Project Overview



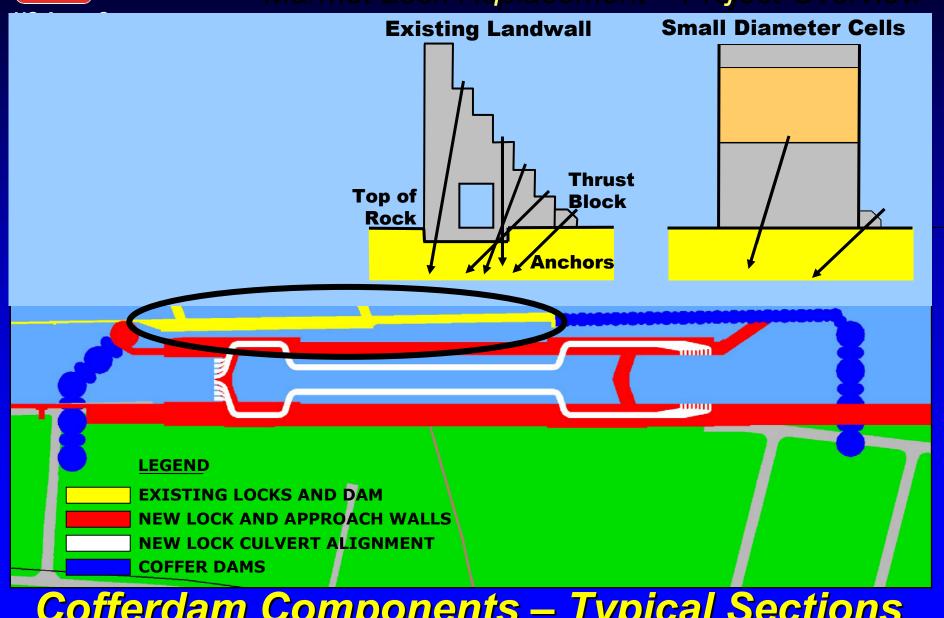








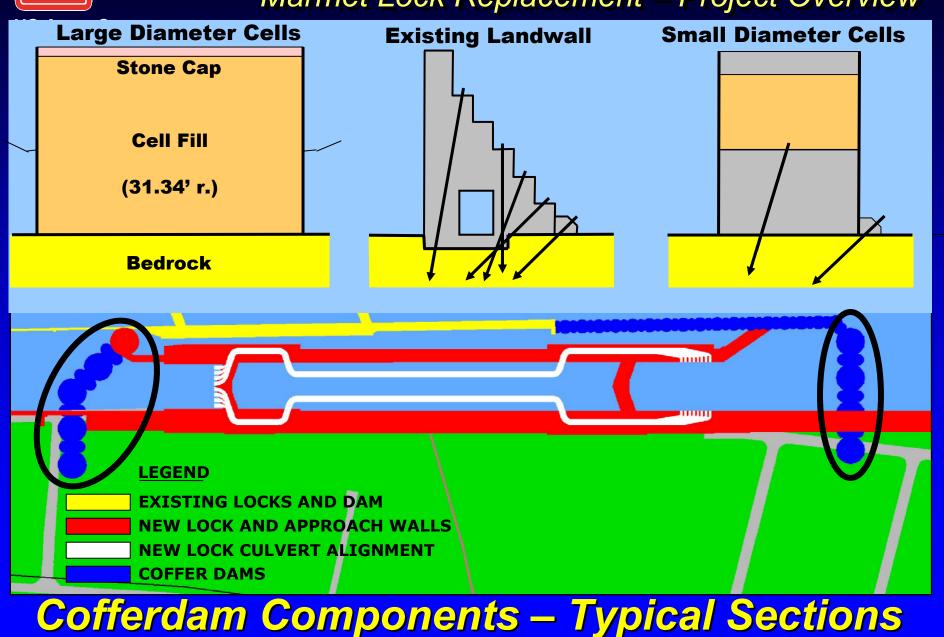




Cofferdam Components - Typical Sections

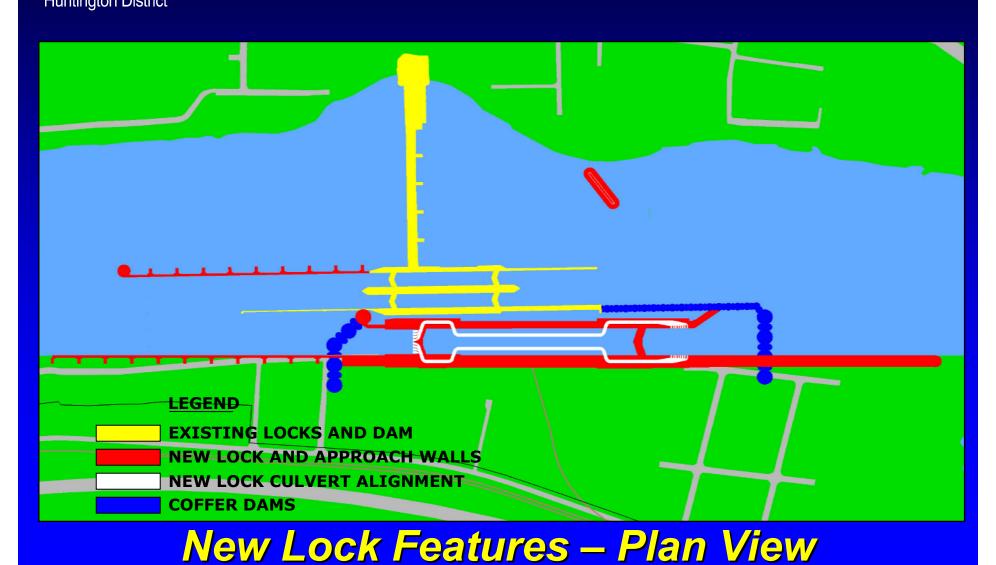






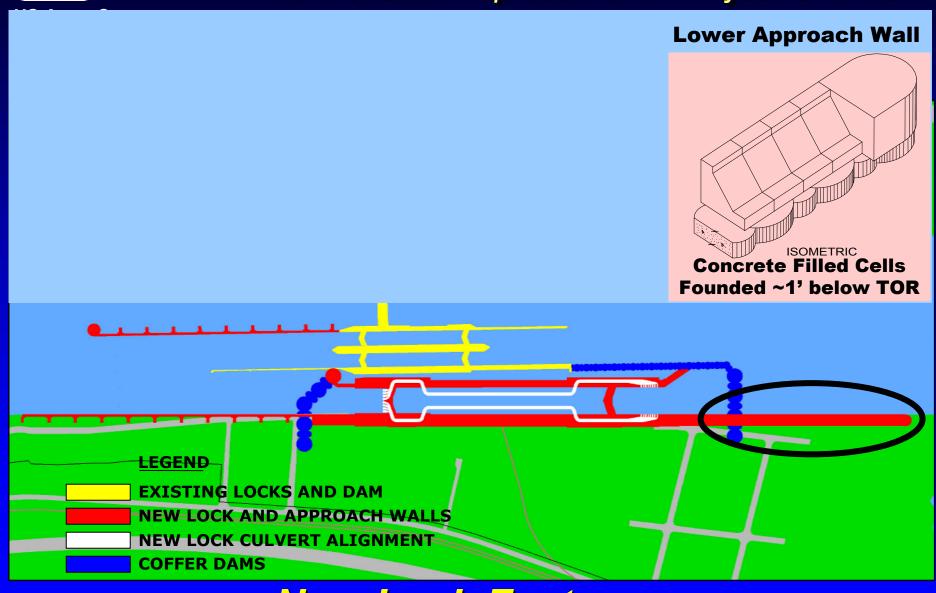


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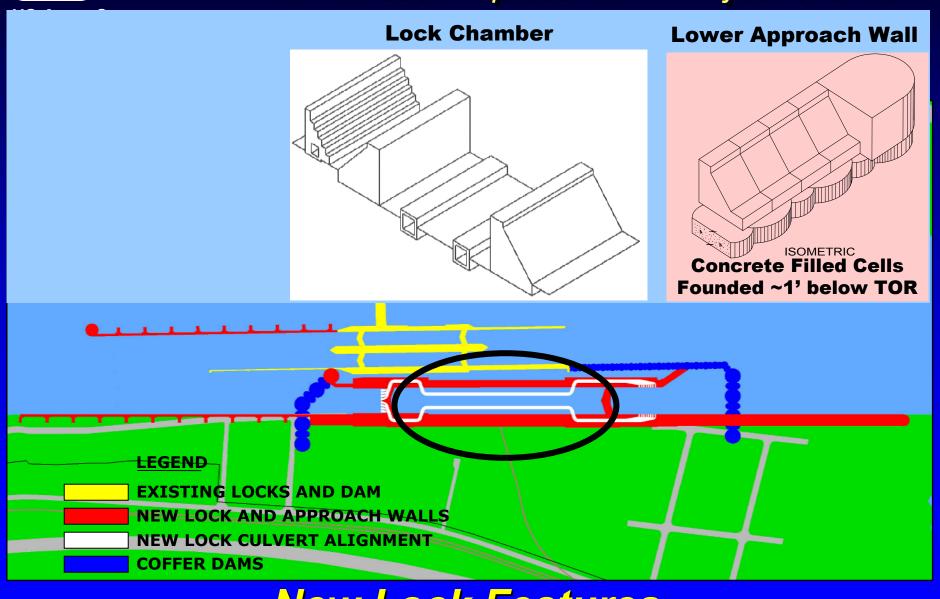






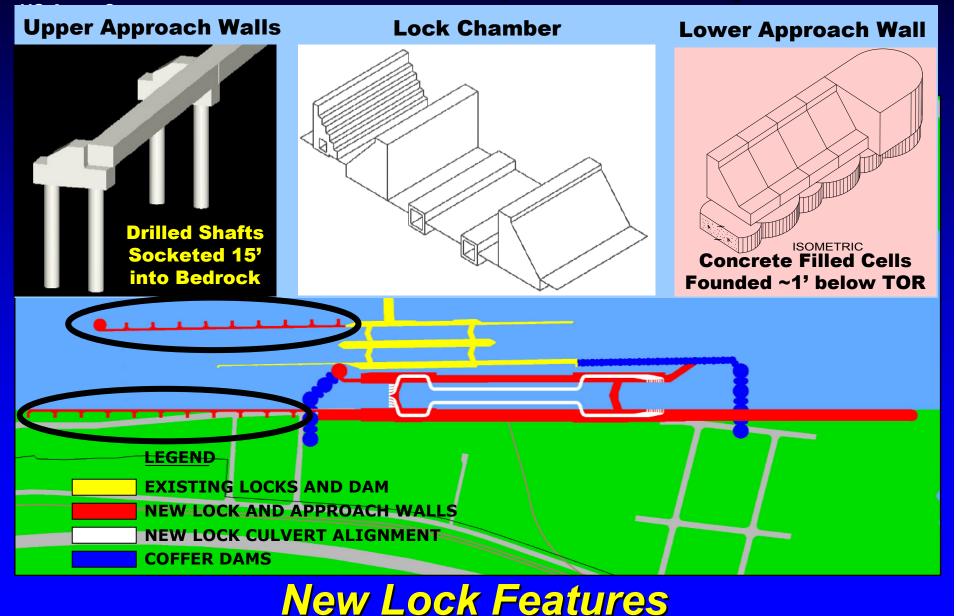
New Lock Features





New Lock Features





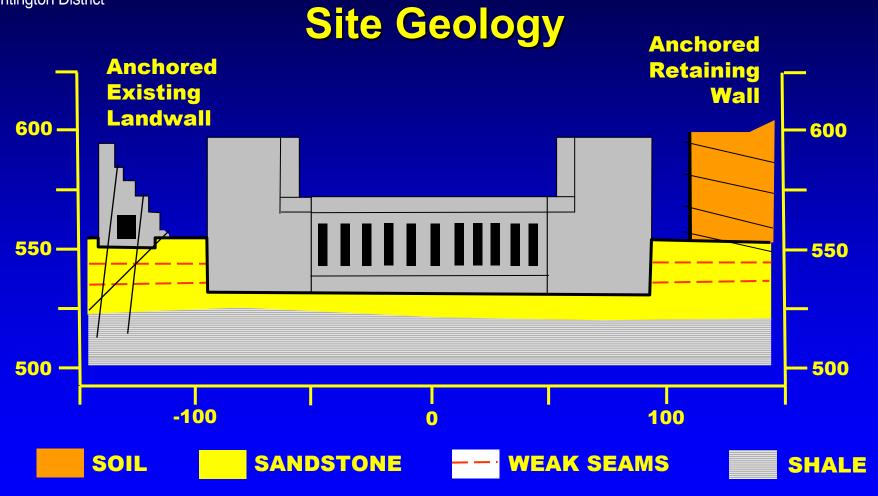


Site Geology

- Relatively flat top of rock surface
- Sedimentary rock of the Pennsylvanian-aged Kanawha Formation
- Nearly horizontal bedding. Discovered low angled (5 - 10°) dip to the Northwest during construction with localized stratigraphic dip
- Rock Units:
 - Sandstone member (23 to 43 feet thick)
 Shale member (19 to 33 feet thick)
- Sandstone member is competent but interbedded with many thin seams and stringers of coal and shale. Discovered to be continuous in excavated sidewalls during construction.



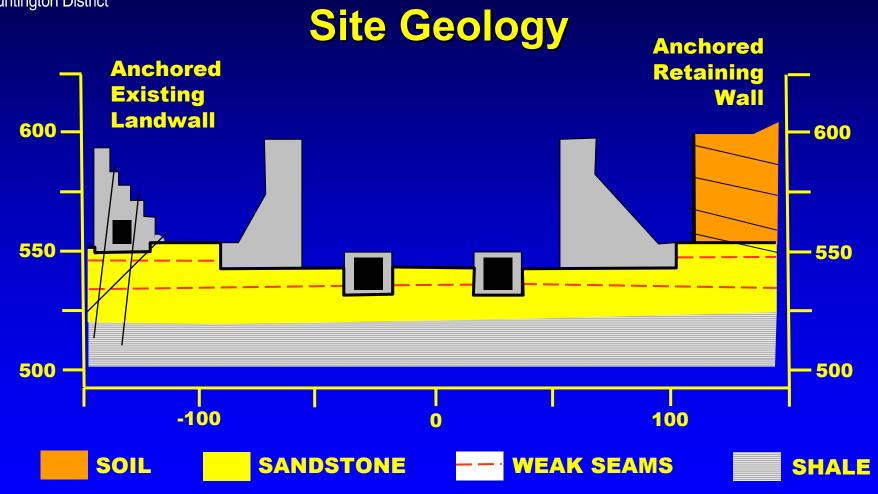
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Geologic Cross Section - Upper Miter Gate



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Geologic Cross Section – Chamber Monoliths



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Marmet Lock Replacement – Project Overview

Site Geology



4.1 4.2 4.3 4.4 4 5 4.6 4.7 4.8 4.9

- Light gray
- Moderately hard to hard
- Medium to fine grained
- Average unconfined compressive strength8,442 psi

Sandstone Member



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Marmet Lock Replacement – Project Overview

Site Geology



#F 4-1 4-2 4-3 4-4 4-5 4-6 4-7 4-8 4-9 5 4-5 8-5 8-5 (-5 5)

- Gray to dark gray
- Moderately hard to soft
- ♦Silty
- Average unconfined compressive strength6,678 psi

Shale Member



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Rock Mechanics Testing

- Design Sliding Friction Strength:phi angle = 30°, cohesion = 4psi
- ◆Cross Bed Shear Strength: phi angle = 45°, cohesion = 73psi





Typical Sandstone Member Bedding Plane



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Rock Mechanics Testing



Design Sliding Friction Strength: phi angle = 23°, cohesion = 0psi



Thin Shale and Coal Seams within Sandstone



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Main Topics of Discussion

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Marmet Lock Replacement – Main Topics

Main Topics of Discussion

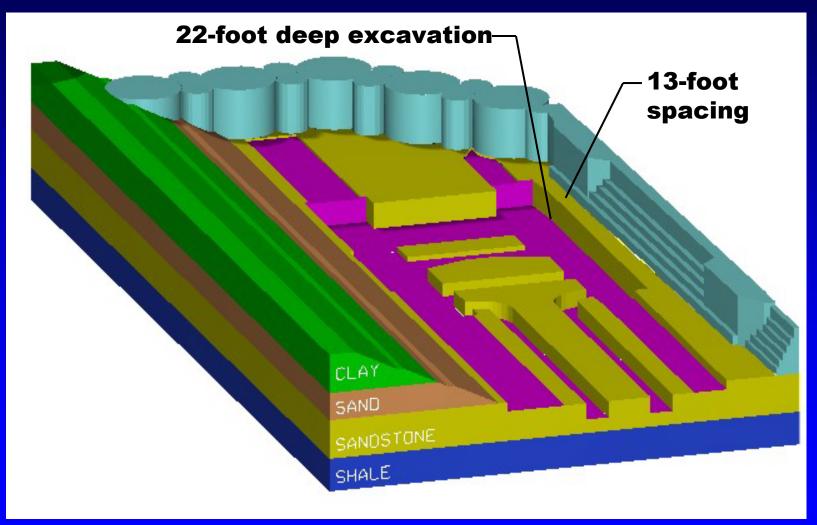
2. DEEP SEATED SLIDING

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Marmet Lock Replacement - Deep Seated Sliding

Design Concerns



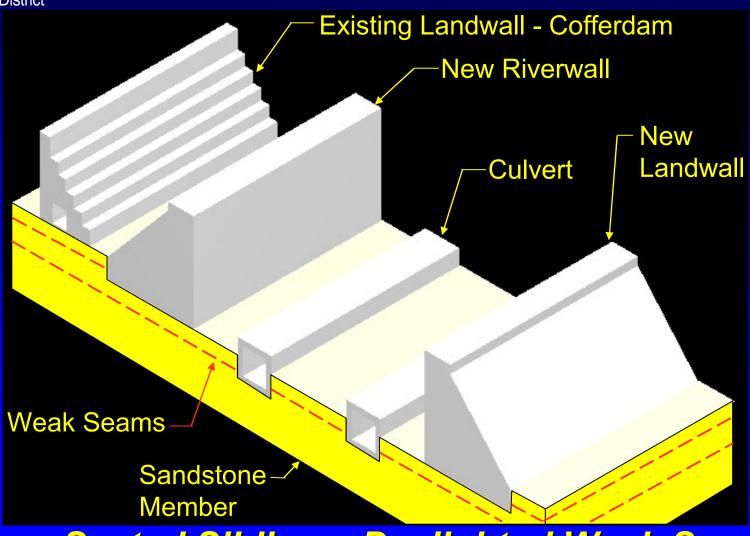
Excavation Adjacent to Structures – 3D view



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Marmet Lock Replacement - Deep Seated Sliding

Design Concerns



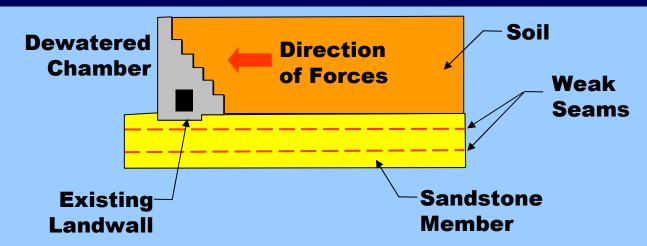
Deep Seated Sliding - Daylighted Weak Seams



Marmet Lock Replacement - Deep Seated Sliding

Design Concerns

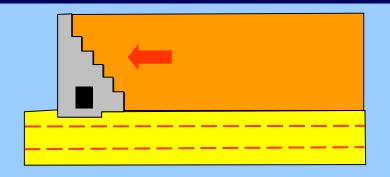
Pre-Construction Condition



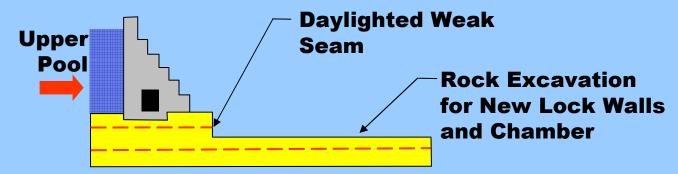


Design Concerns

Pre-Construction Condition

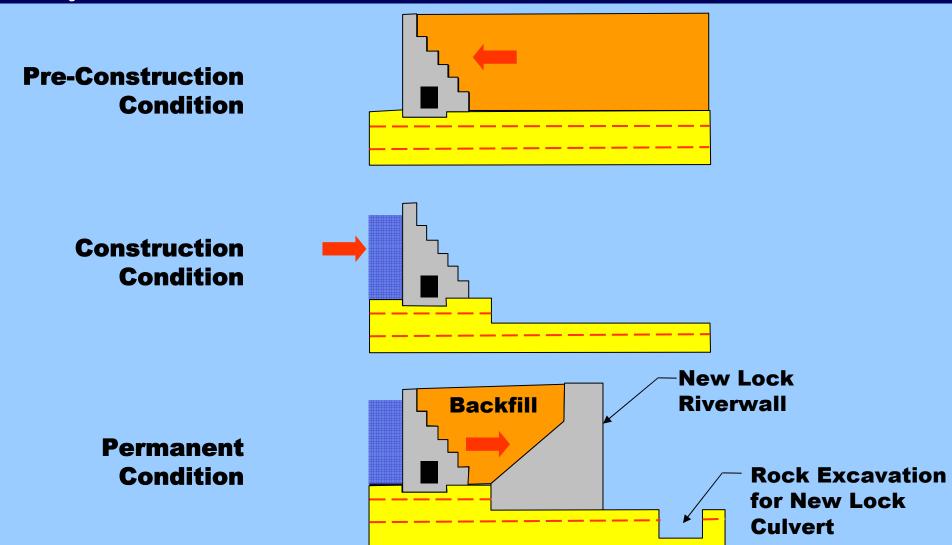


Construction Condition



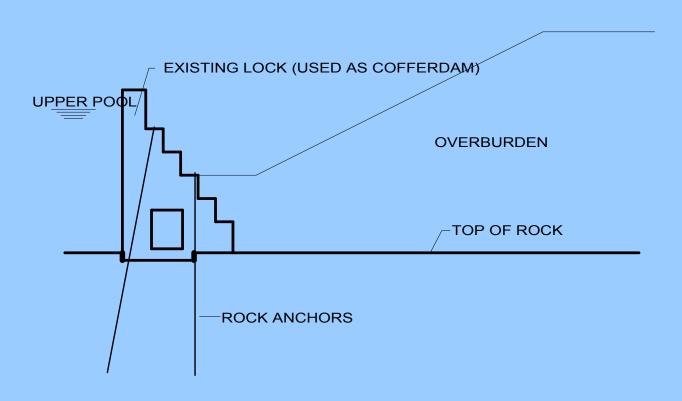


Design Concerns





Design Concerns

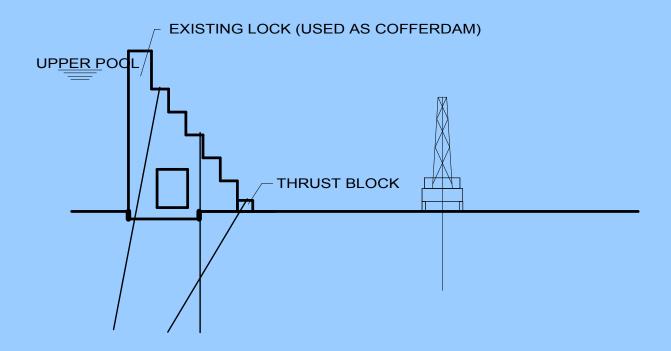


STAGED ANCHORING DURING OVERBURDEN EXCAVATION Analyzed for coal/shale seam at foundation (phi=23°, c=0psi)

INSTALL INSTRUMENTATION



Design Concerns

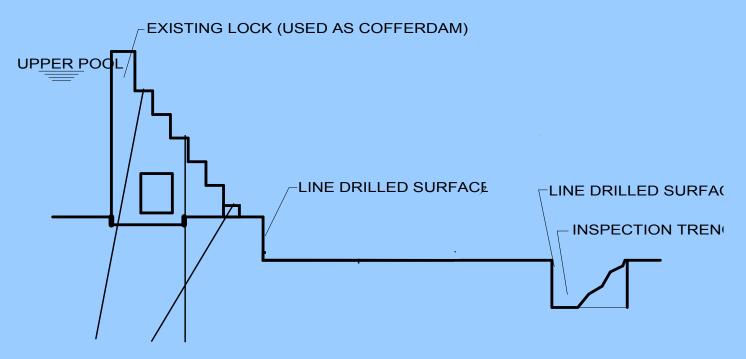


THRUST BLOCK ANCHORED PRIOR TO ROCK EXCAVATION Analyzed for daylighted horizontal fault gouge (phi=13°, c=0psi)

SUBSURFACE EXPLORATION PERFORMED



Design Concerns



COMPLETE EXCAVATION FOR CULVER/INSPECTION TRENCH
ESTABLISH PRESENCE AND EXTENT OF WEAK SEAMS – NEW LOCK
DETERMINE CORRECIVE FOUNDATION TREATMENT – NEW LOCK



Manual Instruments

Manually Read Instruments

- Portable Inclinometers
- Settlement And Alignment Pins
- Joint Monitoring Pins
- Saw Cuts

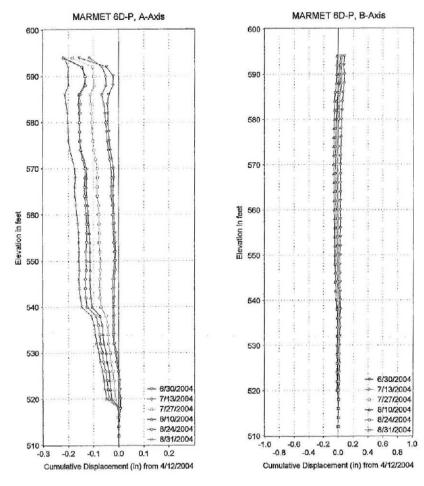


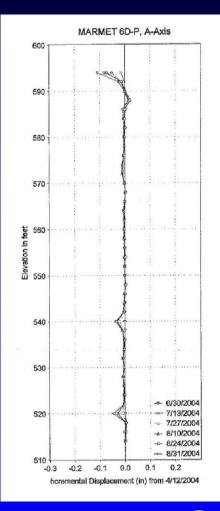
Deep Seated Sliding Occurs

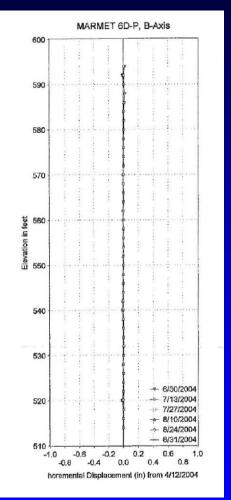
- ◆ Initial Downstream Movement
 - August 2004 Movement Occurred Along Two Weak Planes
 - El 540 +/- Where There Is A Series Of Thin Seams Of Carbonaceous Shale And Coal Within An Otherwise Generally Competent Sandstone Formation
 - El 520 +/- Where There Is A Discontinuous Thin Seam Of Clayey Material Near The Bottom Of The Sandstone Unit And An Underlying Shale Formation. This Seam Is Slightly Lower Than Any Required Excavation







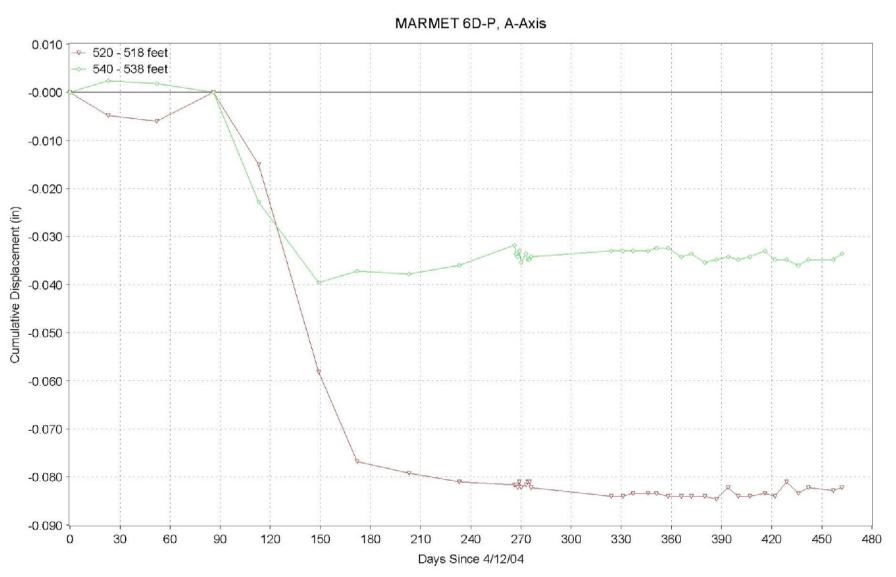




Initial Movement – Cell 6D August 2004



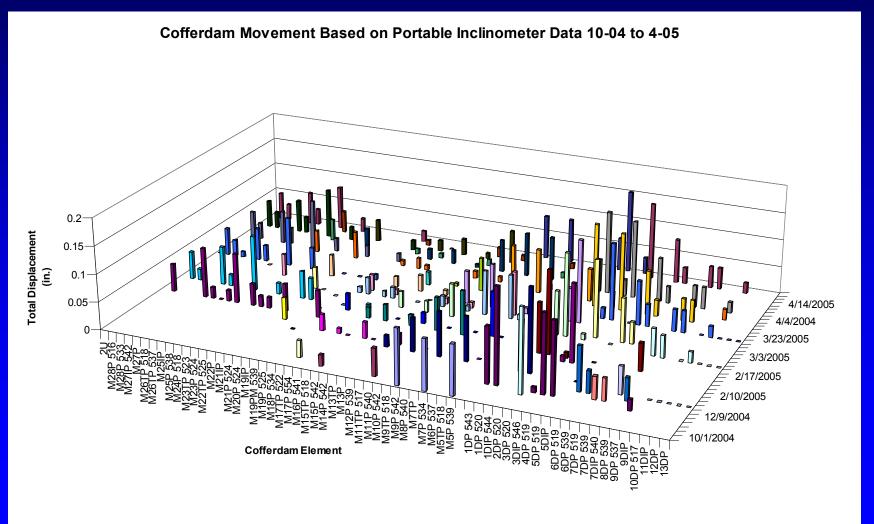
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Portable Inclinometer Displacement





E.A.P. Response

Recommended Actions Of The PDT

- Increase Frequency Of Instrumentation Readings And Evaluation Of Data
- Reanalyze Additional Failure Planes Within The Rock Foundation
- Add Additional Upstream Anchors
- Add Additional Inclinometers Of Greater Depth And At Additional Locations
 - **Obtained Contingency Anchors**



EAP Movement Action Levels

0.20 in. Displacement

- Mobilize Contractor to Install "Contingency" Anchors
- Accelerate Placement of Concrete on Deep Excavation Monoliths
- Partial Flood and/or Backfill Chamber Work Area
- Limit Rock Excavation

0.25 in. Displacement

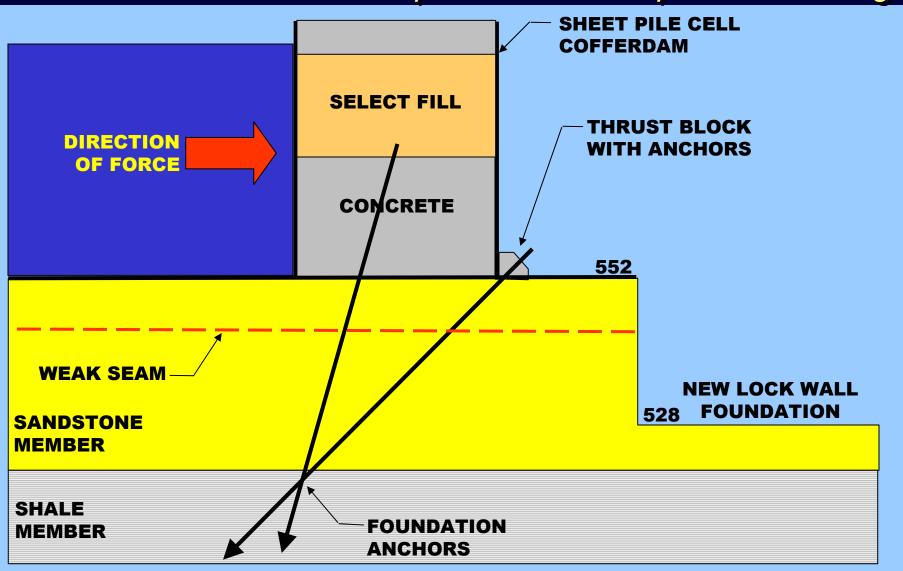
- All Non-remediation Work Stopped and Evacuated
- Work Area Partially or Completely Flooded and/or Backfill

0.35 in. Displacement

- Failure Immenent
- Evacuate Excavation



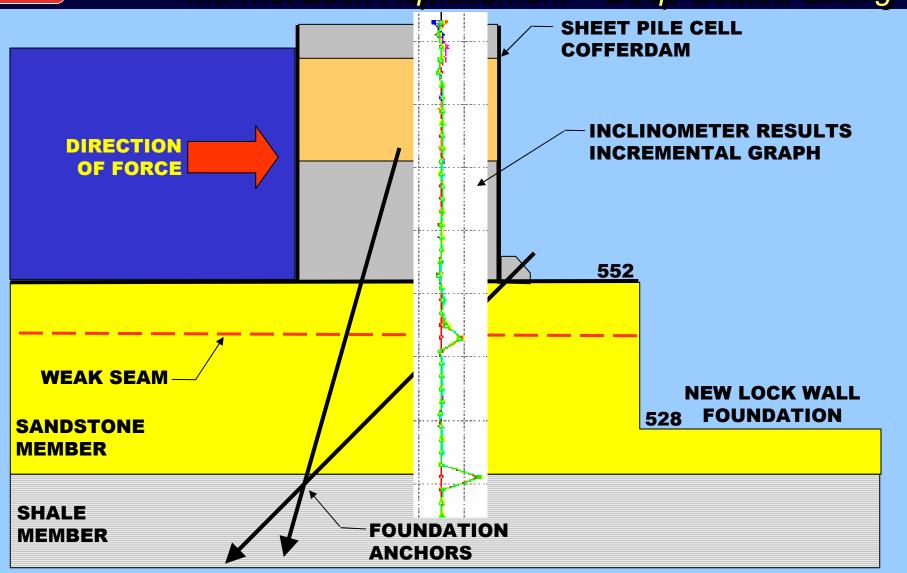




Cofferdam Foundation Movement - Section



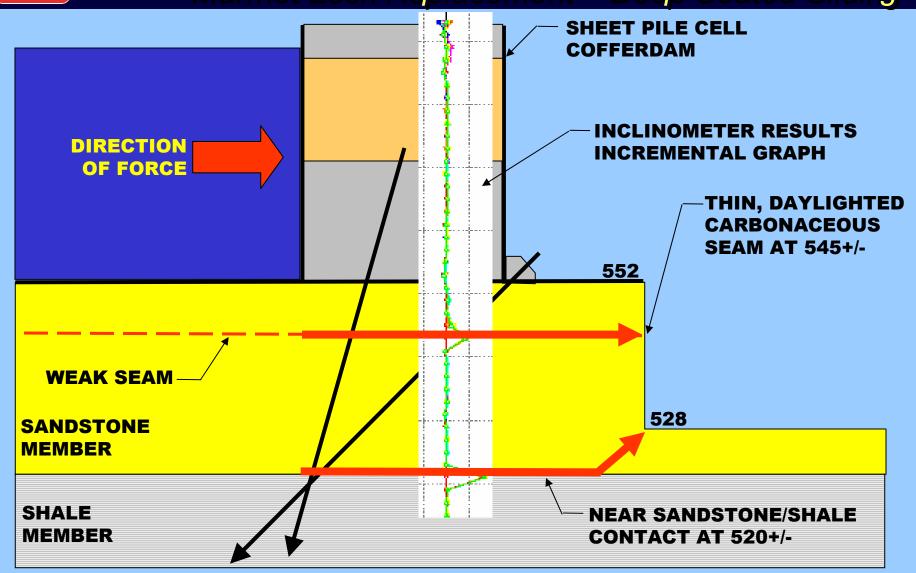




Cofferdam Foundation Movement - Section



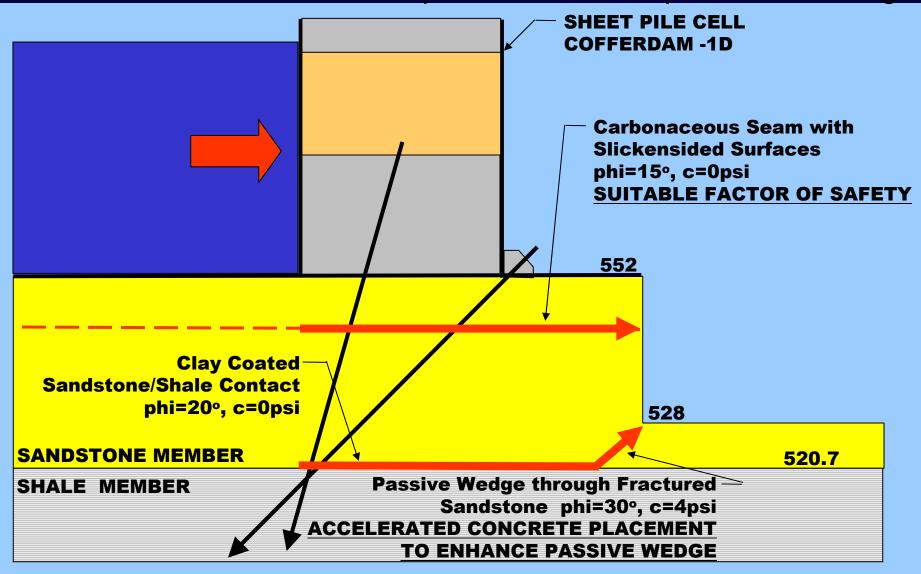




Cofferdam Foundation Movement - Section

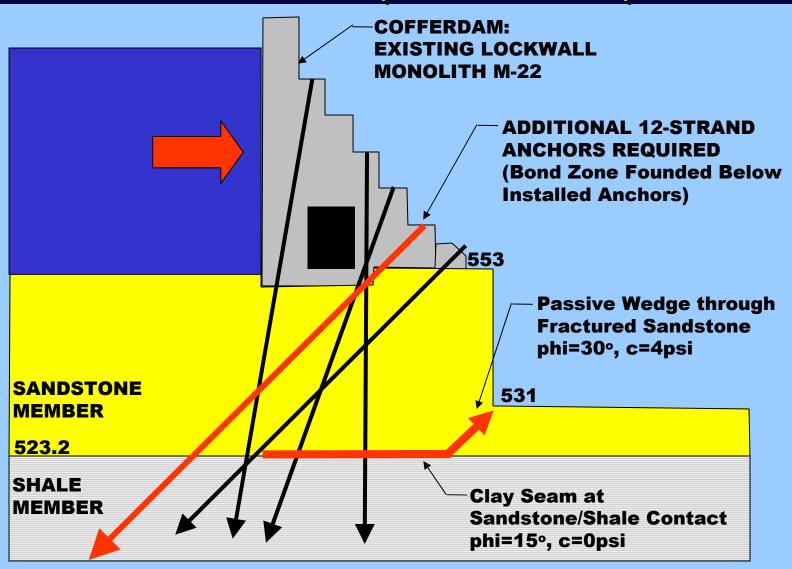






Cofferdam Foundation Movement — Cell 1D





Cofferdam Foundation Movement — M-22

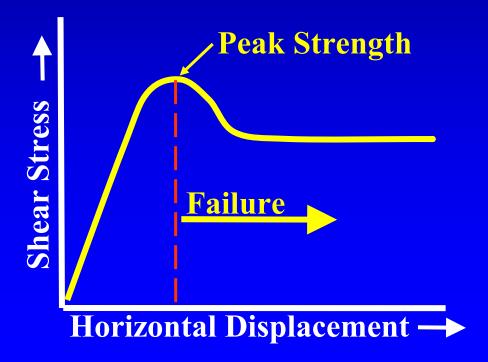


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Cofferdam Foundation Movement

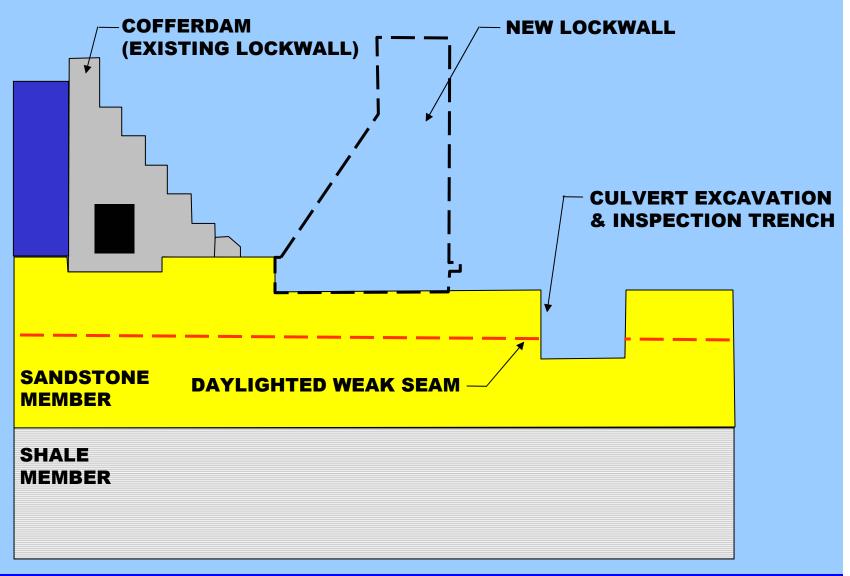
- Some displacement is required to engage rock mass shear strength
- Establish how much movement is acceptable



SCALE EFFECT OR LENGTH OF SHEARED BLOCK WAS TAKEN INTO CONSIDERATION

0.35 INCHES
ESTABLISHED AS
APPROACHING
FAILURE



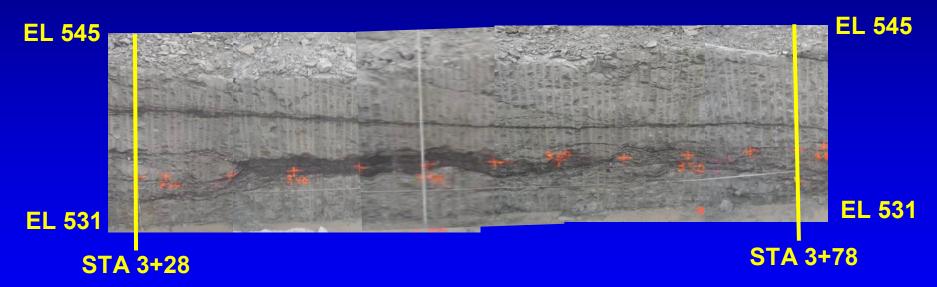




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Inspection Trench Sidewall - Photo

MONOLITH R-15A

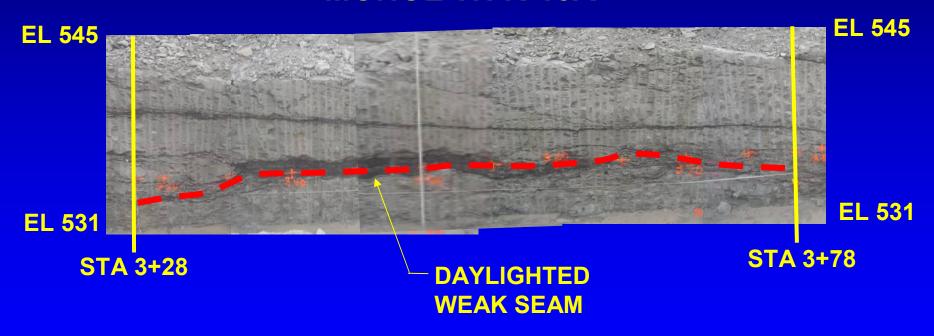




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Inspection Trench Sidewall - Photo

MONOLITH R-15A

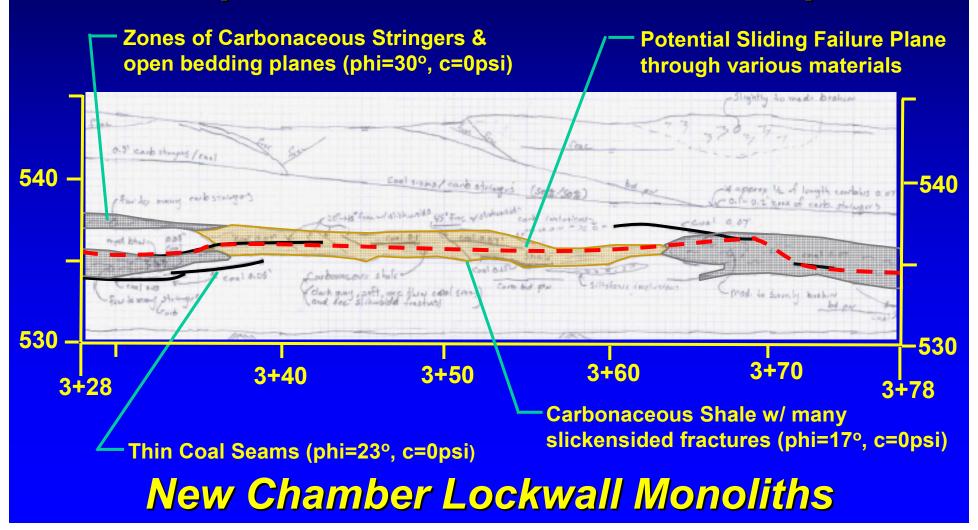




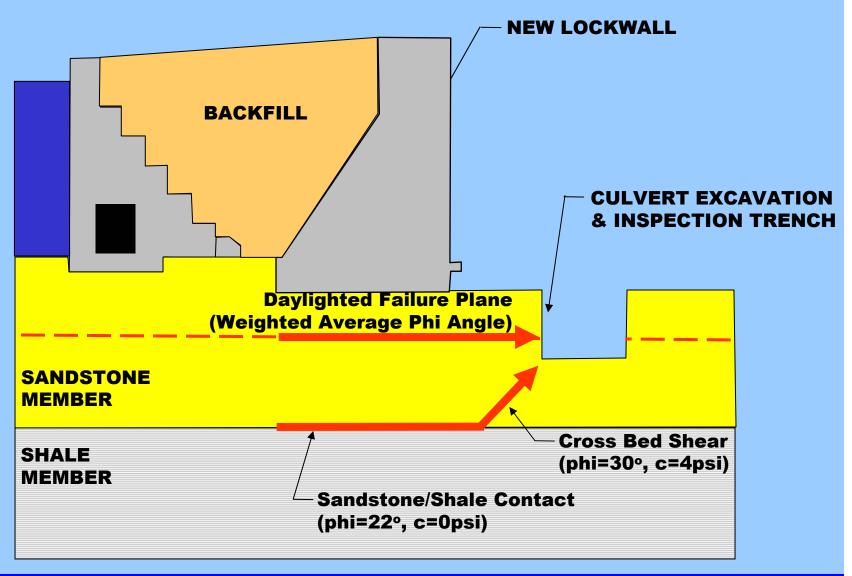
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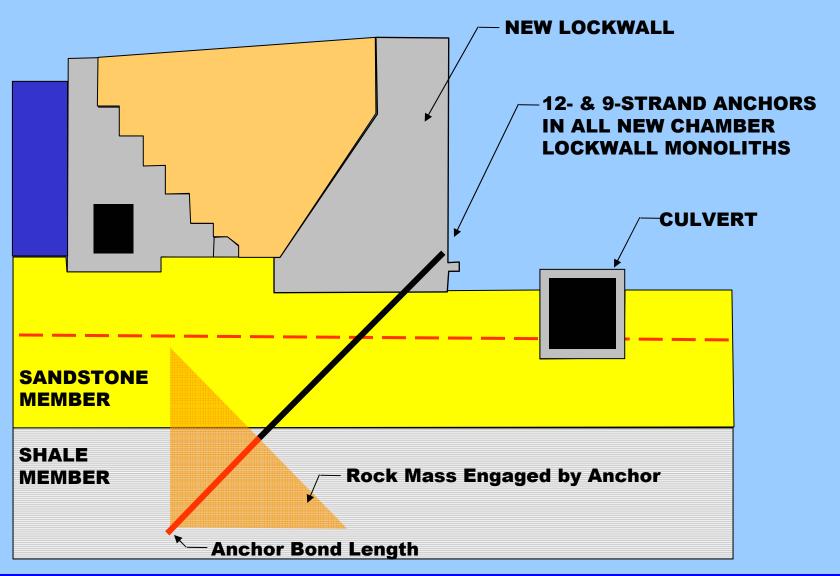
Inspection Trench Sidewall - Map













Marmet Lock Replacement - Main Topics

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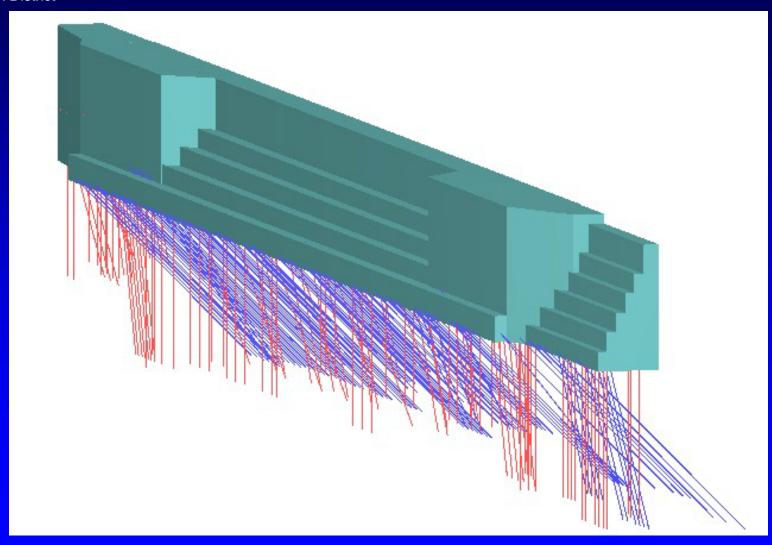
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US Army Corps Over 560 anchors required

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Anchor Drilling







Anchor Alignment Specifications

- Initial Alignment
 - Within 0.5° of Specified Azimuth
 - Within 0.5° of Specified Inclination
- Drilling Alignment
 - Within 0.5° Starting Azimuth
 - Within 0.5° Specified Inclination
 - Less than 1 in. Deviation per 10 ft. of Drilling



Anchor Alignment Testing





Tropari

Maxibor



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Anchor Tensioning





Anchor Tensioning Loads

| | GUTS | Design Load | Lock Off Load (70% GUTS) |
|-----------|----------|----------------|-----------------------------|
| 9 Strand | 527 kips | 316 kips | 369 kips |
| 12 Strand | 703 kips | 422 kips | 492 kips |
| 15 Strand | 879 kips | 527 kips | 615 kips |



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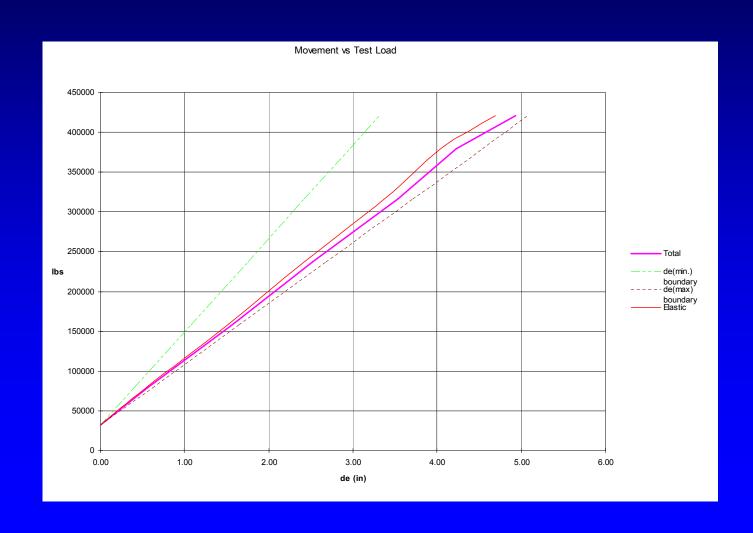
Specified Anchor Testing

- Followed PTI
- Performance Tests
 - One Anchor per Cofferdam Element
- Proof Tests
 - All Remaining Anchors in each Cofferdam Element
- Extended Creep Tests
 - Due to Shale in Bond Zone
 - Conducted on All Performance
 Tested Anchors



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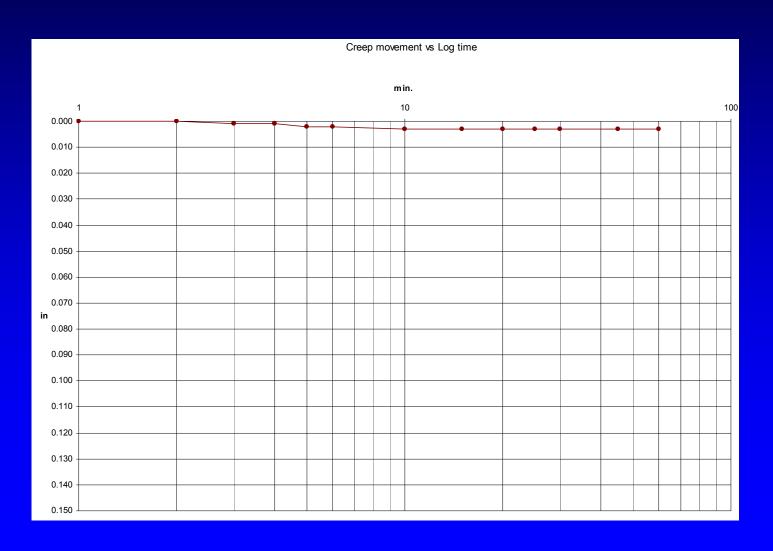
Performance Test Results





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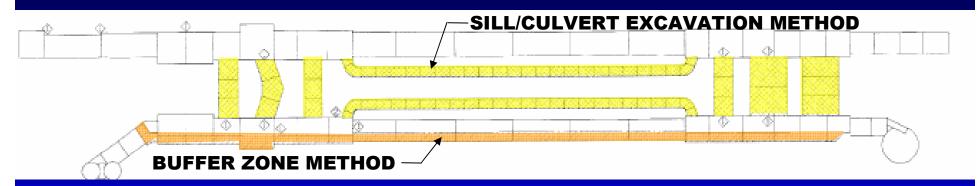
Creep Test Results

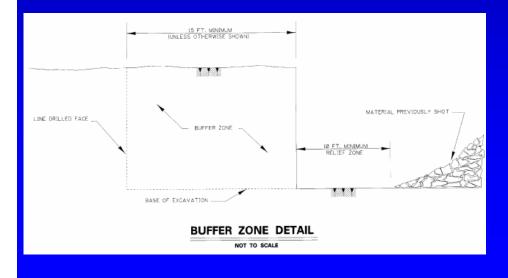




Marmet Lock Replacement – Rock Excavation

Excavation Restrictions



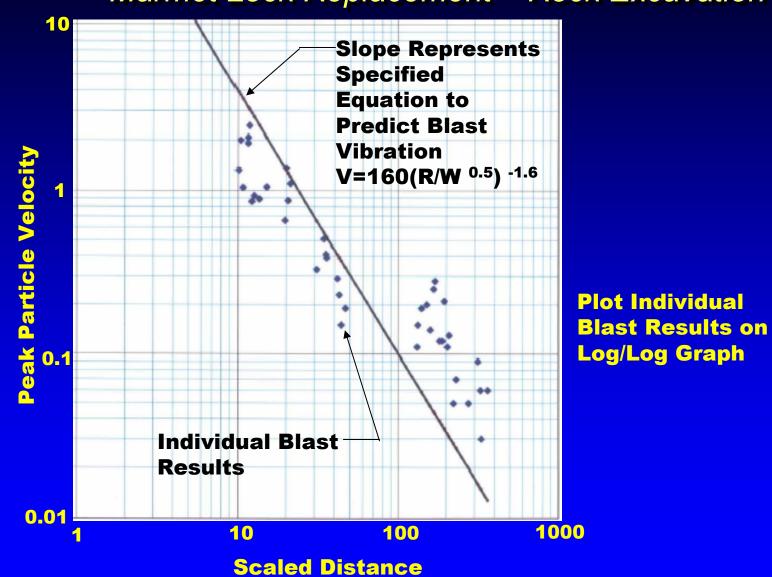


- Pre-blast Survey of Community
- 50' Maximum Blasting Dimension Along Axis
- Buffer Zone and Sill Excavation Methods
- ◆ 3 in/sec Peak Particle Velocity at Nearest Structure
- Line Drilled Perimeters



Marmet Lock Replacement – Rock Excavation



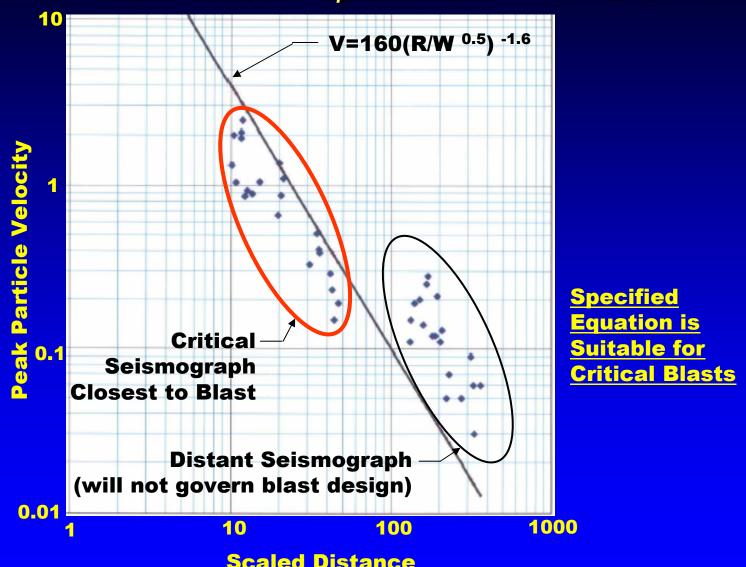


Evaluating Specified Vibration Equation



Marmet Lock Replacement – Rock Excavation





Scaled Distance

Evaluating Specified Vibration Equation



Marmet Lock Replacement – Rock Excavation

Line Drilled Perimeter





Marmet Lock Replacement – Rock Excavation

Production Shot





Foundation Preparation Challenges

- Lenticular Shale Bedding
- Blast Damage
- Thin Bedded Carbonaceous
 Laminations and Tree Fossils
- Artesian Joints



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Lenticluar Beds





Lenticular Shale Bed Plan View







Blasting Damage Types

- Over Break Behind Line Drilled Faces
- Sub Drilling



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Over Break Behind Line Drilled Face





Over Break Behind Line Drilled Face





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Sub Drilled Shot Hole Damage





Shot Hole Damage





Shot Damage Post Foundation Prep.





Artesian Joints







Joints Excavated







Pipes Placed





Concrete Placed





Carbonaceous Laminations and Tree Fossils





Marmet Lock Replacement – Drilled Shafts

Upstream Approach Walls



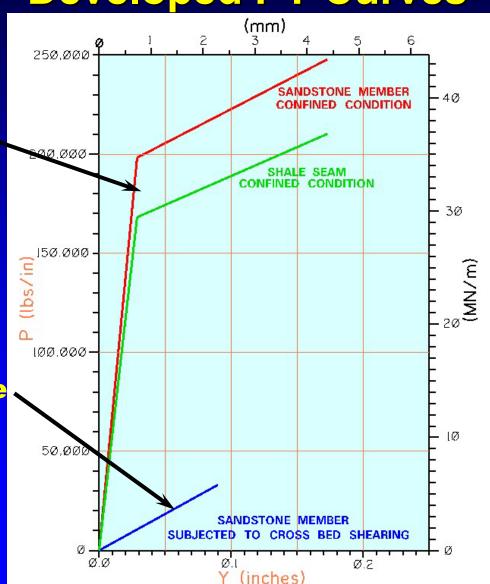


Utilized Borehole Jack Results

Utilized Passive Wedge Results

Marmet Lock Replacement – Drilled Shafts

Developed PY Curves





Drilled Shaft Drill Rig







Auger Drill Bits







Drilled Shaft Core Bit







Rebar and Concrete Placement

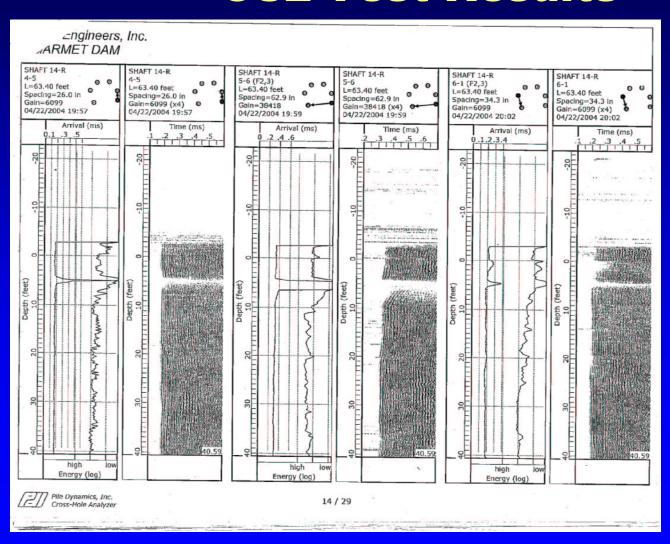






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Drilled Shaft CSL Test Results





Drilled Shaft Low Density Concrete

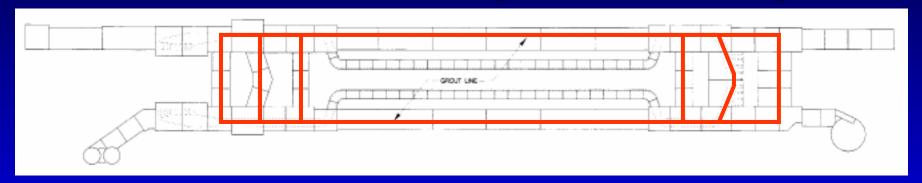


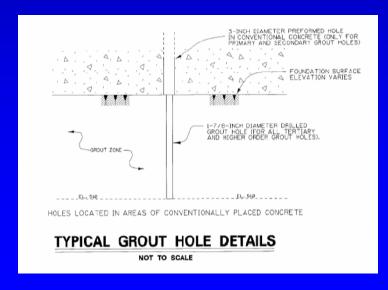


Marmet Lock Replacement - Drilling & Grouting

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Foundation Drilling and Grouting





- Grout Curtain Extends to Elevation 510
- 10' Spacing Between Primary and Secondary Holes
- Optional Tertiary and Higher Order Holes
- All Holes Pressure Testing
- Neat Cement Grout



Marmet Lock Replacement - Conclusion

QUESTIONS AND ANSWERS

Contact Information

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